



# SPECIFICATION 333002: WASTEWATER FORCE MAINS AND APPURTENANCES

## PART 1.0 GENERAL

### 1.1 DESCRIPTION

- 1.1.1 The following specification covers the design, installation, inspection, testing, and acceptance of wastewater force main systems. Construction consists of furnishing all labor, equipment, tools, appliances and materials for performing all operations necessary for the construction and installation of wastewater force mains, including all wastewater, sewage sludge, raw sewage and effluent piping, valves, valve boxes, casings, and appurtenances, complete and ready for operation, as indicated on the construction drawings and described herein. All requirements of the Florida Department of Environmental Protection shall be complied with in addition to the criteria contained within.
- 1.1.2 The Developer/Contractor shall furnish to the County a two year warranty on the materials, fabrication, and workmanship of any and all pipe and fittings furnished and installed. Warranty period shall commence upon written acceptance of the particular component or appurtenance by County for ownership and operation.
- 1.1.3 All Construction plans, project submittals and record drawings shall comply with the requirements of Section 1 and Section 2 of the Hillsborough County Public Utilities Department (PUD) Technical Manual.

### 1.2 REFERENCE DOCUMENTS

- American Association of State Highway & Transportation Officials (AASHTO)
- American National Standard Institute (ANSI)
- American Petroleum Institute (API)
- American Society of Mechanical Engineers (ASME)
- American Society for Testing and Materials (ASTM)
- American Water Works Association (AWWA)
- Florida Department of Transportation (FDOT)
- Manufacturers Standardization Society of the Valve and Fittings Industry (MSS)
- NSF International (NSF)

### 1.3 SHOP DRAWINGS AND SUBMITTALS

- 1.3.1 For County run projects, shop drawings and related manufacturer's product certification shall be made in accordance with the General and Special Conditions of the Contract for approval prior to purchase or fabrication of the material by the manufacturer. The following items which require shop drawings are brought to the Contractor's attention. The list may not include all items for which shop drawing submittals are required to meet the requirements of the project.
- 1.3.1.1 Detail drawings of all classes of pipe, joints, and fittings.
- 1.3.1.2 Detail Drawings of restrained and flexible joints, including test reports to confirm thrust restraint capacities and restraining mechanism application.
- 1.3.1.3 Pipeline laying schedule, for pipelines greater than 12-inch in diameter, tabulated and referenced to construction line and grade controls shown on plans, with station, offset,



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and elevations. References shall be provided for pipe fittings, valves, connections and other important features of the pipeline.

- 1.3.1.4 Detail drawings showing location/plan views of all Jack and Bore Pits (Specification 330524) and all Horizontal Directional Drill Pits (Specification 330523).
  - 1.3.1.5 Line Connections.
  - 1.3.1.6 Valves and Valve Boxes.
  - 1.3.1.7 All Appurtenant Items.
  - 1.3.1.8 Contractor's plan to record and electronically monitor every fusion joint for all fusible PVC installed. The plan shall include the names of the fusion technicians and certification(s), a description of the equipment to be used, and logged information for each joint shall include the proposed heat plate temperatures, and fusion heating/cooling times and pressures, etc.
  - 1.3.1.9 Contractor's flushing and testing plans for all force main pressure piping supplied.
- 1.3.2 Certification and test reports for the materials, manufacturing, and testing of the types of pipe shall be performed and furnished by the pipe manufacturer/supplier in accordance with the latest standards of the industry as described in Part 1.2.
- 1.3.3 **Shop drawing submittals for items listed in Appendix B, the Approved Products List, do not require material certification.**
- 1.3.4 Submit a copy of any design exception prior to installation. Design exceptions are issued by the Utility Design Section Manager. Any deviation from the specifications requires a design exception.

### 1.4 RELATED WORK

- All Sections of Division 03
- All Sections of Division 33
- Hillsborough County Public Utilities Department (PUD) Technical Manual
- Hillsborough County Utility Accommodation Guide and Rights of Way Use Procedures Manual
- Hillsborough County Transportation Technical Manual

## PART 2.0 DESIGN

### 2.1 FLOW CRITERIA

- 2.1.1 Flow estimates for design shall be calculated based on full or projected ultimate development. The average daily flow (ADF) for single family or master-metered residences shall be the per unit demand factors contained in the most current Hillsborough County Utility Rate Resolution. Industrial and commercial design flows for sanitary wastewater shall be no less than the values given in Table 1 of the County's Utility Rate Resolution.
- 2.1.2 Wastewater gravity collection systems, pumping stations, and force mains shall be designed for average daily flow times the appropriate peaking factor. Refer to Section 4 of the PUD Technical Manual for flow criteria and peaking factors.
- 2.1.3 Force mains shall be designed to maintain a minimum velocity of two feet per second (fps). For design friction losses in force mains calculated using the "Hazen-Williams" formula, the value for "C" shall not exceed "C=100" for unlined iron or steel pipe, and "C=120" for smooth pipe materials such as PVC and lined ductile iron.



## **2.2 MINIMUM SIZE**

- 2.2.1 The minimum size force main constructed within the County road right-of-way or dedicated easements shall not be less than four inches in diameter.
- 2.2.2 Force mains shall be sized to carry the full development peak flow (ref. Part 2.1) from all connected pumping stations within the designated stations service area. Each force main system should be capable of transporting the peak flow from each pump station operating simultaneously without producing excessive pressure, i.e., not to exceed 100 feet Total Dynamic Head (TDH) anywhere in the system.

## **2.3 LINE ROUTING**

- 2.3.1 Force mains for a residential or commercial subdivision shall be routed within County Road Right-of-Way.
- 2.3.2 All designs require the EOR to have Level "A" SUE work (locate) performed for all points of connection. Level "A" SUE shall comply with the definition by ASCE 38-02 and adopted by FDOT.
- 2.3.3 Lines crossing arterial roads, collector roads, and any single access entry to a subdivision or traditional neighborhood development shall be cased. The casing shall extend two feet beyond the back of curb.
- 2.3.4 All crossings of arterial and collector roads shall be by jack and bore, unless an alternate installation method is approved by Right-of-Way Permitting, or the Jurisdictional Authority for the road.
- 2.3.5 For projects where the proposed improvement is over existing wastewater lines, all pipe material not meeting the currently approved specifications will require replacement and relocation.
- 2.3.6 Minimum force main line clearance from the property line is five feet.

## **2.4 DEPTH OF COVER**

- 2.4.1 The depth of cover, as measured from finished grade to the top of the pipe shall be not less than 48 inches.
- 2.4.2 When automatic air release valves are required, the depth of cover of the entire line must be increased to a minimum of 52 inches (enough to maintain the valve vault flush with the existing or proposed grade). See Specification 333006, Exhibit S-12A through S-12C for ARV details and required depths of bury.
- 2.4.3 For road widening projects where the road improvement is built over existing force mains, cover shall be 48 inches (minimum) from top of pipe to finished road surface. If depth cannot be maintained, the force shall be relocated.

## **2.5 HORIZONTAL SEPARATION**

- 2.5.1 Wastewater force mains shall be laid at least 10 feet horizontally from any existing or proposed potable water main. A three foot horizontal separation shall be maintained between a wastewater force main and all other pipelines. The distance shall be measured face to face.



- 2.5.2 When the required horizontal separations cannot be maintained, a design exception must be obtained from the Utility Design Section Manager prior to construction commencement.

## **2.6 VERTICAL CROSSINGS**

- 2.6.1 Vertical separation between wastewater force mains crossing other pipelines/utilities shall be a minimum vertical distance of 18 inches between the outside of the other pipelines/utilities and the outside of the wastewater force main. This shall be the case where the other pipeline is either above or below the wastewater main.
- 2.6.2 Potable water main crossings below wastewater line(s) should be avoided whenever possible. Crossing(s) shall be arranged so that the wastewater main joints will be equidistant and as far as possible from the water (potable, reclaimed, or storm) main joints.
- 2.6.3 When the required vertical separations cannot be maintained, a design exception must be obtained from the Utility Design Section Manager prior to construction commencement.

## **2.7 VALVES**

- 2.7.1 Isolation Valves
- 2.7.1.1 Valves and roadway boxes shall be provided for all branch connections (three valves on a tee, four valves on a cross) or other locations, as required to facilitate operation of the system. All valves shall be installed at the tee, cross, or point of connection.
- 2.7.1.2 The maximum allowable distance between in-line (isolation) valves, required to be shut down for repair work, shall not exceed 1,000 feet.
- 2.7.1.3 Valves shall be readily accessible, and located in an area not subject to flooding. Valves shall not be located below the top of bank within a storm water “ditch”, or within a swale.
- 2.7.1.4 Valves shall not be located in ADA (American with Disabilities Act) ramps, or in curbs.
- 2.7.2 Air Release Valves
- 2.7.2.1 Air release valves shall be specified at high points where air can accumulate in new or altered force mains.
- 2.7.2.2 Air release valves shall not be installed in, or adjacent to driveways.
- 2.7.2.3 Construction plans and record drawings shall include air release valve stationing and a detailed plan and profile view.
- 2.7.2.4 Where automatic ARVs are required, the depth of bury for the line must be increased to ensure the valve vault is flush with grade. See Specification 333006, Exhibits S-12A through S-12C for ARV details and the required depth of bury.
- 2.7.2.5 Automatic air release valves shall not be used in situations where flooding of the valve manhole may occur.

## **2.8 PIPE, FITTINGS, AND APPURTENANCES**

- 2.8.1 All force main piping shall be Polyvinyl Chloride (PVC) or ductile iron (DI) except as specified in Part 2.8.2. and 2.8.4. Above ground discharge piping at pump stations shall comply with the requirements in Hillsborough County Technical Specification 333003. Pipe larger than 24 inches in diameter shall have material approved on a case-by-case basis.
- 2.8.2 All fittings shall be ductile iron and comply with the requirements of Part 3 herein.



- 2.8.3 The minimum design working pressure shall be 200 psig, with a laying length of 20 feet
- 2.8.4 Aerial Crossing pipe, pipe attached to a bridge or drainage structure, shall be ductile iron. Above grade piping shall be Class 53 (minimum).
- 2.8.5 All buried ductile iron pipe and fittings shall be polywrapped.
- 2.8.6 The force main system is a closed system. The end of pipes shall be capped or plugged.
- 2.8.7 **Restrained Joints**
  - 2.8.7.1 Restrained joints shall be installed wherever force main pipe alignment changes direction.
  - 2.8.7.2 The length of pipe to be restrained and actually restrained shall be noted on the design and record drawings, respectively. Restrained joints shall be designed with a trench type (per ANSI/AWWA C600 or C605) no higher than Type 3, a safety factor no less than 1.5, and a design pressure no less than 150 psi.
  - 2.8.7.3 Shop drawings from the manufacturer shall be submitted to and approved by the Engineer prior to actual construction. Refer to Part 1.3.
  - 2.8.7.4 Thrust blocks are not allowed in the Hillsborough County wastewater system
  - 2.8.7.5 When Exhibit S-8A is used the Design Engineer of Record must fill out the entire restraint table as applicable for their project

**2.9 CONNECTIONS TO COUNTY SYSTEMS**

All connections to existing County systems shall be approved by the County. All connections to existing force mains shall be made under the direct supervision of Hillsborough County PUD. At all new points of connections, a tee or cross with the appropriate isolation valves shall be installed.

**PART 3.0 PRODUCTS**

**3.1 MATERIAL**

- 3.1.1 The following table lists the allowable pipe materials for various sizes of wastewater force main pipe:

<b>Diameter</b>	<b>Material</b>	<b>General Specifications</b>
4" to 12"	PVC	AWWA C900 DR 18 AWWA C909 Class 200
16" and Greater	PVC	AWWA C905 DR 18
4" and Greater	DIP	ANSI/AWWA C151/A21.51, CL 50 (minimum) with internal coating

- 3.1.2 All pipe, fittings and appurtenances shall be supplied in accordance with the pre-approved material list in Appendix B.
- 3.1.3 Alternative materials may be considered by Hillsborough County if proper testing documentation, performed by recognized industry authorities, is submitted for review to the PUD Product Review Committee prior to commencement of design. Refer to Section 1 of the Hillsborough County PUD Technical Manual for submittal procedures. Tests on alternative materials should be at least as rigorous as testing conducted by ASTM, AWWA and ANSI. A letter of variance must be issued by PUD prior to commencement of design incorporating any alternative material.



## **3.2 POLYVINYL CHLORIDE PIPE AND FITTINGS**

- 3.2.1 All PVC pipe shall be GREEN.
- 3.2.2 PVC pressure pipe shall have the same O.D. as ductile iron pipe and be compatible for use with ductile iron fittings. The pipe shall conform to pipe dimension ratio (DR) of 18, shall have a minimum working pressure of 200 psi, and a nominal laying length of 20 feet.
  - 3.2.2.1 Pipe (four-inch through 12-inch in diameter) shall conform to the requirements of ANSI/AWWA C900 or C909, except that the pipe does not have to be NSF approved.
  - 3.2.2.2 Pipe 16-inch in diameter and larger shall meet all requirements of AWWA C905, except for NSF approval.
- 3.2.3 All PVC shall be formulated for sunlight exposure and shall pass the impact strength test as described by ASTM D2444, latest revision, using Tup A with impact level of 94 ft.-lbs.
- 3.2.4 Fittings: Fittings for PVC pipe shall be ductile iron mechanical joint and comply with the requirements of Part 3.3.4.
- 3.2.5 Joints
  - 3.2.5.1 PVC pipe shall have provisions for expansion and contraction provided in the joints.
  - 3.2.5.2 All non-fused joints shall be designed for push-on makeup connection. A push-on joint shall be an elastomeric gasket bell end coupling manufactured as an integral part of the pipe barrel consisting of an integral wall-thickened expanded bell end section with a ring groove to retain an elastomeric sealing ring of uniform cross-section as approved in Appendix B for PVC pipe.
  - 3.2.5.3 Restrained Joints: See Specification 333006, Exhibits S-8A & S-8B for restrained joint details. See Appendix B for pre-approved products. The length of pipe to be restrained shall be noted on the Construction Drawings.
- 3.2.6 Fusible PVC
  - 3.2.6.1 Pipe shall be provided with plain ends. The ends shall be square to the pipe and free of any bevel or chamfer. There shall be no bell or gasket of any kind incorporated into the pipe.
  - 3.2.6.2 Fusible PVC shall be manufactured in a standard 20-foot, 30-foot or 40-foot nominal length.
  - 3.2.6.3 Fusible PVC pipe lengths shall be assembled in the field with butt-fused joints. The Contractor shall follow the pipe supplier's written guidelines for this procedure, using only demonstrated qualified fusion technicians.

## **3.3 DUCTILE IRON PIPE AND FITTINGS**

- 3.3.1 For Ductile Iron (DI) pipe the following shall apply:
  - 3.3.1.1 Pipe shall conform to the requirements of ANSI/AWWA C151/A21.51, "*Ductile-Iron Pipe, Centrifugally Cast, For Water*", Class 50 (minimum).
  - 3.3.1.2 Pipe coatings shall comply with ANSI/AWWA C151/A21.51, section 4.3.3 (protective interior ceramic epoxy coating), and 4.3.1 (external asphaltic coating) for buried pipe. The internal coating shall comply with Appendix B. Internal coatings do not require NSF approval.
  - 3.3.1.3 Joints shall be mechanical or push-on joints, unless otherwise specified herein.
  - 3.3.1.4 Refer to Specification 333003 for pump station discharge piping.



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- 3.3.2 The weight and class designation shall be legibly marked on the exterior surface of every pipe. Manufacturer's code or serial number shall be provided on the bell of each pipe joint.
- 3.3.3 Ductile iron pipe shall be marked with a two-inch wide, green stripe. Striping shall be 4.5-mil thick (minimum) adhesive backed tape inscribed with the words "wastewater" wrapped around the pipe in a continuous spiral with bands 12 inches to 18 inches apart for the length of each pipe section. The tape shall be secured to each end by wrapping it back upon itself.
- 3.3.4 Fittings
  - 3.3.4.1 Ductile iron fittings shall be mechanical joint and restrained with a minimum pressure rating of 250 psig, and shall conform to the requirements of ANSI/AWWA C153/A21.53, "*Ductile-Iron Compact Fittings for Water Service*".
  - 3.3.4.2 Coatings shall comply with 4.4.6 (protective interior ceramic epoxy coating) and 4.4.2 (external coating) of ANSI/AWWA C153/A21.53. Ductile iron fittings and coatings shall be as approved in Appendix B.
- 3.3.5 Mechanical Joints
  - 3.3.5.1 Mechanical joints (MJ) consisting of bell, socket, gland, gasket, bolt and nuts shall conform to ANSI/AWWA C111/A21.11, "*Rubber-Gasket Joints for Ductile-Iron Pressure Pipe and Fittings*".
  - 3.3.5.2 Bolts shall be high strength low alloy steel. T-head type having hexagonal nuts.
  - 3.3.5.3 Bolts and nuts shall be machined true and nuts shall be tapped at right angles to a smooth bearing surface.
- 3.3.6 Push-on Joints: Single seal gasket push-on type joints shall conform to the requirements of ANSI/AWWA C111/A21.11 and Appendix B.
- 3.3.7 Restrained Joints
  - 3.3.7.1 Restrained joints (RJ) shall be of the types fabricated by the various pipe manufacturers and not the type that requires field welding or grooves cut into the pipe barrel for restraint.
  - 3.3.7.2 The restraining joints for mechanical joint (MJ) fittings and valves shall conform to the requirements of ANSI/AWWA C111/A21.11 and Appendix B. See Specification 333006, Exhibit S-8A.
  - 3.3.7.3 Restrained joints (both manufacturer supplied and field lock gaskets) shall have the bell of the pipe marked in red. Wrap the bell with vinyl, adhesive red marking tape.
- 3.3.8 Aerial Crossing Pipe, Hangers, and Accessories
  - 3.3.8.1 Pipe designated for use in aerial crossings and/or attachment to bridge or drainage structures shall be ANSI/AWWA C151/A21.51, Class 53 (minimum) ductile iron pipe.
  - 3.3.8.2 For aerial crossings, pipe length shall correspond to "Long Span Pipe", D.I.P. restrained joint.
  - 3.3.8.3 Pipe joints shall consist of a mechanical joint-flange (MJ-FLG) or flange-plain end (FLG-PE), and flange restrained (FLG-RJ). The flange joint end shall be equipped with an O-ring gasket.
  - 3.3.8.4 The DIP shall be ordered to fit the job. No field cuts will be allowed.
  - 3.3.8.5 All ductile iron pipe shall have an interior ceramic epoxy coating as specified in Appendix B. Exterior coating shall be field applied, color coded green, and comply with the pre-approved products specified in Appendix B.
  - 3.3.8.6 Hangers and Accessories



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- a) Anchor Bolts: Anchor bolts shall be 316 stainless steel, installed in accordance with the Construction Drawings, and utilizing non-shrink grout.
- b) Roller Stands: Roller stands and roller axles shall be 316 stainless steel.
- c) Insulated Pipe Rollers: Pipe support rollers shall be constructed of dielectric synthetic resin.
- d) Link Seal and Sleeve-Seals shall be modular mechanical type, consisting of interlocking synthetic rubber links shaped to continuously fill the annular space between the pipe and wall opening. Links shall be loosely assembled with bolts to form a continuous rubber belt around the pipe with a pressure plate under each bolt head and nut.
- e) Hangers shall be made in accordance with standard practice MSS SP-58, and in full compliance with Federal Specification WW-H-171E and the ANSI/ASME Code for pressure piping B 31.1.
- f) Polywrap/Polyethylene Encasement: All buried ductile iron pipe and fittings shall be encased in polyethylene in accordance with ANSI/AWWA C105/A21.5, "*Polyethylene Encasement for Ductile-Iron Pipe Systems*". Polyethylene shall be eight mils thick.

### 3.4 GASKETS

Pipe and fitting gaskets, conforming to ANSI/AWWA C111/A21.11, shall be made of viton (fluorocarbon elastomer), EPDM (ethylene propylene diene monomer) or SBR (styrene-butadiene rubber). Material selection shall be dependent upon service and soil conditions.

### 3.5 VALVES AND APPURTENANCES

#### 3.5.1 General

- 3.5.1.1 All valves shall be the manufacturer's standard design for the service intended and shall be cast with the manufacturer's name and pressure rating on the body, and if applicable, the valve type, size, and flow direction arrow.
- 3.5.1.2 Valves shall open left (counter clockwise), when viewed from the top. The operating nut, or hand wheel, shall have an arrow cast in the metal indicating the direction of opening.
- 3.5.1.3 See Appendix B for pre-approved valves. Valve ends shall be flanged for all above ground installations and shall follow the general requirements as specified in Specification 333003 "Wastewater Pumping Stations."

#### 3.5.2 Plug Valves

- 3.5.2.1 Plug valves shall be fully bidirectional and meet the requirements of AWWA C517 and C550. Valve ports shall have a minimum 80 percent full pipe area up to 12 inches, and 100 percent full pipe area for 14 inches and greater.
- 3.5.2.2 Valves shall have MJ-RJ ends and shall be furnished complete with joint accessories.
- 3.5.2.3 Valves shall be coated with a fusion bonded epoxy coating (10 mils minimum) applied to both the exterior and the interior surfaces prior to assembly of the valves.
- 3.5.2.4 Valve and gearing shall be rated for a minimum of 150 psi pressure rating. The valves shall provide drip-tight shut off at rated pressure in both directions.
- 3.5.2.5 All external hardware shall be 304 stainless steel.
- 3.5.2.6 The seat end of plug valves shall be installed facing into the direction of flow.

#### 3.5.3 Tapping Valves, Tapping Sleeves & Service Saddles

- 3.5.3.1 Tapping sleeves, tapping crosses, and tapping valves used to make "wet" taps into



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existing mains shall be provided and installed at locations as shown on the Construction Drawings.

3.5.3.2 No size on size taps are allowed.

3.5.3.3 Tapping Valves

- a) Tapping valves shall be gate valves with a mechanical joint outlet, non-rising stem, resilient seat, and with O-ring seals meeting the applicable requirements of ANSI/AWWA C509 or C515, and C550.
- b) All tapping valves shall be coated with a fusion bonded epoxy coating applied to both the exterior and the interior surfaces prior to assembly of the valves.
- c) Tapping valves shall be furnished with a combination flange and mechanical joint for connecting the branch to the main.
- d) Tapping valves shall be specifically designed for pressure tapping with sufficient seat opening to allow full diameter taps to be made.
- e) Tapping valves shall be manufactured with an integral tapping flange having a raised face or lip designed to engage the corresponding recess in the tapping sleeve flange in accordance with MSS SP60.
- f) Tapping valves without the raised face on the tapping flange are not permitted since they do not assure the proper alignment required to prevent damage by a misaligned shell cutter.
- g) The tapping valve shall be considered sacrificial. Once the tap has been made, a plug valve shall be installed for operation/isolation, and the tapping valve shall be locked in the fully open position.

3.5.3.4 Tapping Sleeves and Crosses

- a) Tapping sleeves shall be stainless steel (SS) with wraparound gasket style, MJ split iron units, or fabricated carbon steel units with a fusion-bonded epoxy coating and outlet seal gaskets, and shall be pressure rated listed in 3.5.2.4.
  - 1) SS with wraparound gaskets shall be limited for use on all pipes up to 12 inches in diameter.
  - 2) MJ split iron units shall be limited for use on PVC pipe upto 12 inches in diameter and shall not be used on AC pipe.
  - 3) Fabricated carbon steel units with fusion bonded epoxy coating and outlet seal gaskets shall not be used on AC pipe.
- b) The Contractor shall verify the outside diameter of the existing main before ordering the sleeve.
- c) Tapping sleeves shall be of the mechanical joint type with outlet flange conforming to ANSI B16.1, class 125.

3.5.3.5 Service Saddles

- a) Service saddles as a minimum shall be supplied with double tie straps and shall be fabricated of 316 stainless steel and be suitable for either wet or dry installation.
- b) The sealing gasket shall be the "O-Ring" type suitable for the applicable service. Outlet Flange shall be ANSI B16.1, Class 125 standard.
- c) Tie straps and bolts shall be corrosion resistant 316 stainless steel.
- d) Service saddles shall be as approved in Appendix B.

3.5.4 Air Release Valve Assemblies

3.5.4.1 Air release valves (ARVs) shall be located at high elevation points as indicated on the Construction drawings. The locations may be considered approximate. The actual location of the ARV at the pipeline high points shall be determined in the field during construction and reflected on the record drawings.

3.5.4.2 Automatic air release valves shall operate automatically and be of the type that will



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release air from the line when pressurized and keep air from entering the line when not pressurized. Overall height of the ARV (w/vacuum check) shall not exceed 19-1/6 inches.

- 3.5.4.3 The air release valve assembly shall have a two-inch inlet, 316 stainless steel ball valve, saddle, and 316 stainless steel pipe and fittings.
  - 3.5.4.4 The ball valve shall meet the following criteria:
    - a) It shall be two piece, threaded, stainless steel type 316 meeting ASTM-CF8M A351. The inlet and outlet shall be FIP (NPT). The pipe thread shall be in accordance with ANSI B1.20.1.
    - b) The valve shall be full port design, with a blowout proof stem, 1000 psi (WOG) minimum, and reinforced Teflon seats.
    - c) The ball valves shall conform to API 598, have a stainless steel handle, nuts, and washers, vinyl handle grip, lockable handle and be vacuum rated to 29" Hg (inches mercury).
  - 3.5.4.5 All fittings and piping shall be rated for a minimum working pressure of 150 psi.
  - 3.5.4.6 The automatic air release valves shall be installed in traffic bearing pre-cast concrete vaults with concrete bottoms. See Specification 333006, Exhibits S-12A and S-12B.
- 3.5.5 Valve Boxes
- 3.5.5.1 Cast iron valve boxes with lids shall be provided for all valves installed underground. The valve boxes shall be screw-type adjustable to fit the depth of earth cover over the valve; and designed to prevent the transmission of surface loads directly to the valve or piping. See Specification 333006, Exhibit 9A.
  - 3.5.5.2 Valve boxes shall have an interior diameter of not less than five inches.
  - 3.5.5.3 Valve box extensions shall be installed to reserve a minimum of 50% of the adjustment for the future extension. Extension sections shall only be ductile or cast iron.
  - 3.5.5.4 The valve boxes shall be provided with covers marked "SEWER". The lids shall be painted green, and shall be constructed so as to prevent tipping or rattling.
  - 3.5.5.5 All valve locations shall be identified with a concrete valve pad containing a bronze disc embedded in concrete. See Specification 333006, Exhibits S-10 and S-11.
  - 3.5.5.6 For valves boxes in pavement, the protective concrete collar ring shall be constructed of Type I (3000 psi) concrete. See Specification 333006, Exhibit S-11.

## PART 4.0 CONSTRUCTION

### 4.1 WORK AT HIGHWAY OR RAILROAD CROSSINGS

The construction shall not commence for any work along or within County roads, FDOT highways, or railroad crossing until all permits for the pipeline occupancy have been obtained.

### 4.2 PRECONSTRUCTION PIPE INSPECTION/CERTIFICATION

- 4.2.1 The Contractor shall obtain from the pipe manufacturer a certificate of inspection to the effect that the pipe and fittings supplied for the project have been inspected at the plant and that they meet the requirements of these specifications.
  - 4.2.1.1 For County contracted projects, the Contractor shall submit these certificates to the Project Manager prior to installation of the pipe materials.
  - 4.2.1.2 For developer projects, the Contractor shall submit these certificates to the PUD Inspector prior to the installation of the pipe materials.



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- 4.2.2 Joints or fittings that do not conform to these specifications will be rejected and must be removed immediately by the Contractor.
- 4.2.3 The entire product of any plant may be rejected when, in the opinion of the County, the methods of manufacture fail to secure uniform results, or where the materials used are such as to produce inferior pipe or fittings.
- 4.2.4 For County Contracted projects all pipe and fittings shall be subjected to visual inspection at time of delivery and before they are lowered into the trench to be laid.
- 4.2.5 PVC pipe shall not be off-loaded with chains, wire rope, or other pipe handling implements that may scratch, nick, cut or gouge the pipe. Any scratch or gouge that is greater than 10% of the wall thickness is considered significant and shall be rejected.

### 4.3 INSTALLATION

- 4.3.1 The provisions set forth herein shall be applicable to all underground wastewater piping installations
- 4.3.2 All pipe shall be installed at a minimum depth of four feet (48 inches to the top of pipe) below final grade.
- 4.3.3 All mechanical joints, or connections to pipe, fittings, valves, or apparatus shall be made plumb, so to ensure no negative pressure is placed or potential placed against the joint, or connection, thereby causing a malfunction or failure of the mechanical joint, or connection.
- 4.3.4 All pipe shall be color coded GREEN.
  - 4.3.4.1 Ductile iron pipe shall be marked with a two-inch wide, green, 4.5-mil thick (minimum) adhesive backed tape inscribed with the words "wastewater line," wrapped around the pipe in a continuous spiral with bands 12 to 18 inches apart for the length of each pipe section. The tape shall be secured to each end by wrapping it back upon itself.
  - 4.3.4.2 Polywrap shall be color coded green or clear.
  - 4.3.4.3 PVC pipe shall be green throughout the thickness
- 4.3.5 It shall be the Contractor's and Developer's responsibility to verify all existing conditions and to locate all structures and utilities along the proposed utility alignment in order to avoid conflicts. Where conflicts exist, work shall be coordinated with the facility owner and performed so as to cause minimum interference with the service rendered by the facility disturbed.
- 4.3.6 Facilities or structures damaged shall be repaired or replaced immediately at the Contractor's or developer's expense. The repair or replacement shall be in conformance with current standard industry practices, and according to the direction of the owner of such facility and approved by the County.
- 4.3.7 See Specification 333006, Exhibit S-5 for Jack and Bore details and Exhibit S-6 for Ditch Crossing details.
- 4.3.8 Directional Drill/Jack and Bore: Where open cut is not practical, directional drilling per Specification 330523, or jack and bore per Specification 330524, shall be used. All directional drill, and jack and bore, locations shall be indicated on the Construction drawings, and approval from PUD is required prior to starting construction.



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- 4.3.9 No pipe shall be laid when the trench conditions or the weather is unsuitable for such work.
- 4.3.10 The wastewater force main system is a closed system. The ends of pipes shall be capped or plugged.
- 4.3.11 Lined DI pipe and fittings shall be installed within one year of lining. The date of lining is the first set of numbers in white on the interior of the pipe or fitting.
- 4.3.12 Polyvinyl chloride pipe may be damaged by prolonged exposure to direct sunlight. The Contractor shall take necessary precautions during storage and installation to avoid this damage. Pipe shall be stored under cover and sufficient backfill shall be placed to shield it from the sun as the pipe is installed.
- 4.3.13 All pipe crossing driveways and landscape areas is to be installed by directional drill unless otherwise noted on the construction drawings. All pipe crossing arterial and collector roads shall be installed by jack and bore.
- 4.3.14 All non-metallic (PVC) pipe shall be installed with two insulated 10 gauge solid copper or copper clad steel core locating wires with color coded coating (green) and attached at 10:00 and 2:00. Wire for direction drill applications shall be copper clad "hard drawn" steel core with a minimum breaking strength of 1000 pounds.
  - 4.3.14.1 Wires shall be attached using minimum two inches wide duct tape. Tape shall be at every joint and at four to five-foot spaced intervals.
  - 4.3.14.2 Locating wires shall terminate 4 or more inches above the concrete valve pad and shall be folded back inside a three-inch PVC access pipe (see Specification 333006 Exhibit S-9A). A continuity test shall be performed by the Contractor in the presence of the County inspector.
  - 4.3.14.3 In the case where a valve's location falls within paved road, wires shall be continuous to the next valve outside pavement.
- 4.3.15 When fusible PVC is used, Fusion Technicians must be fully qualified by the pipe supplier to install fusible PVC of the type(s) and size(s) being used. Qualification shall be current as of the actual date of fusion performance on the project.
- 4.3.16 Excavation, trenching and backfilling shall be in accordance with the requirements of the applicable portions of these specifications. In addition, all underground facility installations shall comply with the requirements of section 5.4 of the Utility Accommodation Guide.

### **4.4 TRENCH EXCAVATION**

- 4.4.1 All excavations shall be open cut, with banks of trenches kept as nearly vertical as possible and wide enough to allow approximately eight inches of clearance on each side of the pipe.
- 4.4.2 The trench floor shall provide a uniform bearing for each full length of pipe section. Excavate bell holes after trench has been graded. See Specification 333006, Exhibit 15.
- 4.4.3 Perform all excavations of whatever substance encountered to the depths shown or indicated on plans.
- 4.4.4 In the event unsuitable or unstable soil is encountered, remove it to a depth of six inches (minimum) below the bottom elevation of the pipe (12 inches if rock or boulders are encountered) and replace with material meeting AASHTO Soil Classification A-1, A-2, or A-3, as approved by the Project



Manager or Engineer of Record. Reference FDOT Standard Specifications for Road and Bridge Construction Section 125-4.

- 4.4.5 Dewatering: Remove all water from excavations and maintain the excavations free of water while construction therein is in progress. Provide dewatering equipment as necessary to conform to this requirement. Dewatering procedure must meet all regulatory requirements.
- 4.4.6 Protection of Trees: Trenching shall not take place within the root zone of trees with a trunk diameter of six-inch or larger. The root zone shall be defined as the greater of one) the drip line of the tree or two) a circular zone extending outward from the base of the tree a distance equivalent to 1/2-foot for every inch of trunk diameter as measured 4-1/2 feet above natural grade (see Specification 333006 Exhibit S-7). Exotic nuisance species, such as Brazilian Pepper and Melaleuca, are exempt from this protection.

### **4.5 HANDLING AND CUTTING PIPE**

- 4.5.1 Every care shall be taken in handling and laying pipe and fittings to avoid damaging the pipe, scratching or marring machined surfaces, and abrasion of the pipe coating both inside and out.
- 4.5.2 Lined DIP must be handled only from the outside of the pipe and fittings. No forks, chains, straps, hooks, etc. shall be placed inside the pipe and fittings for lifting, positioning, or laying.
- 4.5.3 Care should be taken not to let the pipe strike sharp objects while swinging or being off loaded. The pipe shall not be dropped or unloaded by rolling.
- 4.5.4 Lined DIP should never be placed on grade by use of hydraulic pressure from an excavator bucket or by banging with heavy hammers.
- 4.5.5 Any fitting showing a crack, and any fitting or pipe which has received a severe blow that may have caused an incipient fracture (even though no such fracture can be seen) shall be marked as rejected and removed at once from the work.
- 4.5.6 In any pipe showing a distinct crack and in which it is believed there is no incipient fracture beyond the limits of the visible crack, the cracked portion, if so approved by PUD, may be cut off before the pipe is laid. The cut shall be made in the sound barrel at a point of at least 12 inches from the visible limits of the crack. All cutting shall be done with a machine adapted to the purpose. All cut ends shall be examined for possible cracks caused by cutting.
- 4.5.7 Cutting Pipe: The Contractor shall cut pipe by means of an approved mechanical cutter. The cut shall be perpendicular to the longitudinal axis of the pipe and rough ends or spurs will be satisfactorily removed prior to installation and seating.
- 4.5.8 Manufacturer guidelines for sealing cut ends and repairing field damaged areas of ceramic epoxy lined DI pipe and fittings must be followed.

### **4.6 FUSION PROCESS**

- 4.6.1 Fusible PVC shall be handled in a safe and non-destructive manner before, during, and after the fusion process and in accordance with this specification and the pipe supplier's guidelines.
- 4.6.2 Fusible PVC shall be fused by qualified fusion technicians, as documented by the pipe supplier.



- 4.6.3 Each fusion joint shall be recorded and logged by an electronic monitoring device affixed to the fusion machine.
- 4.6.4 Only appropriately sized and outfitted fusion machines that have been approved by the pipe supplier shall be used for the fusion process. Fusion machines must incorporate the following properties/elements: heat plate, carriage, and data logging device.
- 4.6.5 Other equipment specifically required for the fusion process shall include the following:
  - 4.6.5.1 Pipe rollers used to support the pipe on either side of the fusion machine.
  - 4.6.5.2 A weather protection canopy that allows full machine motion of the heat plate, fusion assembly and carriage.
  - 4.6.5.3 Fusion machine and maintenance manual shall be kept with the fusion machine at all times.
  - 4.6.5.4 Facing blades specifically designed for cutting fusible PVC.

## **4.7 PIPE LAYING**

- 4.7.1 Pipe shall be constructed of the materials specified and as shown on the drawings.
- 4.7.2 Cradle: Upon satisfactory excavation of the pipe trench and completion of the pipe bedding, a trough recess for the pipe bells and joints (or couplings) shall be excavated by hand digging. When the pipe is laid in the prepared trench, true to line and grade, the pipe barrel shall receive continuous, uniform support and no pressure will be exerted on the pipe joints from the trench bottom.
- 4.7.3 Cleanliness: The interior of the pipes shall be thoroughly cleaned of all foreign matter before being gently lowered into the trench and shall be kept clean during laying operations by means of plugs or other approved methods. During suspension of work for any reason at any time, a suitable stopper shall be placed in the end of the pipe last laid to prevent mud or other foreign material from entering the pipe.
- 4.7.4 Gradient
  - 4.7.4.1 Lines shall be laid straight, and depth of cover shall be maintained uniform with respect to finish grade, whether final grading is completed or proposed at time of pipe installation. When a grade or slope is shown on the Construction Drawings, means shall be used by the Contractor to assure conformance to required grade.
  - 4.7.4.2 Any pipe which has its grade or joint disturbed after laying shall be taken up and re-laid.
- 4.7.5 Pipe/Joint Deflection: Whenever it is desirable to deflect pipe, the amount of deflection shall not exceed the following:
  - 4.7.5.1 For pipe joints: 75% of the maximum limit as specified in AWWA C600 (for Ductile iron) or AWWA C605 (for PVC), or the manufacturer's recommendation, whichever is less.
  - 4.7.5.2 For PVC pipe: 75% of the maximum limit as specified in AWWA C605, or the manufacturer's recommendations, whichever is less.
- 4.7.6 Rejects: Any pipe or fittings found defective due to interior or exterior damage shall be immediately removed and replaced with sound pipe or fittings at the Contractor's expense.
- 4.7.7 PVC: All PVC force mains shall have electronic locator wires in accordance with Part 4.3.14, herein. The wires shall be continuous between valves.



- 4.7.8 Any section of pipe already laid which is found to be defective or damaged shall be replaced with new pipe without additional cost to the County.
- 4.7.9 Installation of PVC pipe and fittings shall be in accordance with the installation requirements established by the manufacturer and AWWA M23.

## **4.8 INSTALLING JOINTS**

- 4.8.1 The joints of all pipelines shall be assembled in straight alignment and made tight. The particular joint used shall comply with the requirements of Part 3.2 and 3.3. For County contracted projects, the particular joint used shall be reviewed and approved by the Engineer of Record prior to installation.
- 4.8.2 Ductile iron pipe and fittings can only be pushed when using a restrained joint system that does not allow the spigot to contact the bell shoulder. Pipe may be pulled using restrained joint pipe or restraining gaskets as restraints.
- 4.8.3 Restraining gaskets must never be pushed; nor should pipe be homed all the way to the bell shoulder.
- 4.8.4 All joints on PVC C905 pipe shall be photographed showing proper insertion, axial deflection, and joint location. At a minimum, the Contractor shall provide a digital photograph of each push on and restrained joint using the following criteria.
  - 4.8.4.1 The photograph shall display the date, time and location (either station and offset, or GPS X-Y coordinates). This information can be electronically “marked” on the photo; or physically marked on the pipe; or a “white” board can be used with the information displayed.
  - 4.8.4.2 The picture resolution shall be no less than three megapixels, set at 2048x1536, ISO 200, and fine/superfine to produce a minimum 1.5 megabyte size photo file.
  - 4.8.4.3 Every photo shall be framed in such a manner that its entire width shall be taken up by the joint, and two feet of pipe on each side of the joint.
  - 4.8.4.4 The Manufacturer’s installation reference mark must be visible and centered in the photograph.
  - 4.8.4.5 The photo file format shall be in JPG.
  - 4.8.4.6 The complete set of photos shall be submitted to the Inspector in DVD format.
- 4.8.5 **Mechanical Joints:** All types of mechanical joint pipes shall be laid and jointed in full conformance with manufacturer's recommendations. Torque wrenches set as specified in ANSI/AWWA C111/A21.11, shall be used; or spanner type wrenches not longer than specified therein may be used with the permission of the County. Impact wrenches shall not be used.
- 4.8.6 **Push-On Joints:** Push-on joints shall be made in strict compliance with the manufacturer’s recommendations.
  - 4.8.6.1 Lubricant shall be an inert, non-toxic, water soluble compound.
  - 4.8.6.2 Insert the spigot end into the bell so that it is in uniform contact with the gasket.
  - 4.8.6.3 For PVC pipe, push the spigot until the reference mark on the spigot end is flush with the end of the bell. If the reference mark is not visible after assembly, the joint is to be cut out and reassembled.
  - 4.8.6.4 For DIP, push the pipe until the reference mark on the spigot end disappears into the bell.
- 4.8.7 **Joint Compounds:** Sulfur based joint compounds shall not be used.



- 4.8.8 Restrained Joints shall be provided at all changes in direction, and size changes, of all mains.
  - 4.8.8.1 All pipe and fitting joints shall be restrained as shown on the Construction Drawings, or where in the opinion of the Design Engineer, settlement or vibration is likely to occur.
  - 4.8.8.2 All restrained joints shall be installed in accordance with manufacturer's recommendations.
  - 4.8.8.3 All restrained joints (manufacturer supplied or field lock gaskets) shall have the joint bell marked in red.

## **4.9 INSTALLING APPURTENANCES**

- 4.9.1 Valves and Valve Boxes
  - 4.9.1.1 Valves shall be carefully inspected, opened wide and then tightly closed and the various nuts and bolts shall be tested for tightness, on site, prior to installation.
    - a) Special care shall be taken to prevent any foreign matter from becoming lodged in the valve seat.
    - b) Any valve that does not operate correctly shall be immediately removed and replaced by the Contractor.
    - c) The seat end of plug valves shall be installed facing into the direction of flow.
  - 4.9.1.2 Valves installed below ground shall be identified with a bronze disc imbedded in concrete to identify wastewater valves (see Specification 333006, Exhibits S-10 and S-11).
  - 4.9.1.3 Concrete Valve Pad: Valve boxes outside of paved areas shall be cast in a 3000-psi concrete slab, two-foot by two-foot square and six-inch (minimum) thick. See Specification 333006 Exhibit S-10 for unpaved installations.
  - 4.9.1.4 Valve Boxes: Valve boxes shall be carefully centered over the operating nuts of the valves so as to permit a valve key to be fitted easily to the operating nut.
    - a) Valve boxes shall be set to conform to the level of the finished surface and held in position by a ring of concrete placed under the support flange. See Specification 333006, Exhibits S-10 and S-11.
    - b) The valve box shall not transmit surface loads to the pipe or valve.
    - c) Care shall be taken to prevent earth and other material from entering the valve box.
    - d) Any valve box which is out of alignment or whose top does not conform to the finished ground surface shall be dug out and reset.
    - e) Before final acceptance of the work, all valve boxes shall be adjusted to finish grade and valve box extensions shall be installed to reserve 50% of their total adjustment for future extension. Extension sections shall be ductile or cast iron only.
    - f) The operating nut should not exceed 36 inches below finished grade. However, if conditions require the operating nut to exceed 36 inches, then an extension, mechanically attached to the valve, shall be added, and the top of the extension shall not exceed 12 inches below finished grade. See Specification 333006, Exhibit S-9B.
    - g) The valve boxes shall be provided with covers marked "SEWER" and shall be so constructed as to prevent tipping or rattling. The valves shall be identified with a bronze disc embedded in concrete.
    - h) Valve boxes shall not be installed in ADA ramps, or in curbs.
- 4.9.2 Air Release Valves
  - 4.9.2.1 Construction plans and record drawings shall include air release valve stationing on both the plan and profile views.
  - 4.9.2.2 The location of ARVs indicated on Construction plans are approximate. The actual placement of the air release valves shall be determined in the field and shall be located



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at the high points as established during installation of the force main and as approved by Project Manager.

- 4.9.2.3 Automatic air release valves shall be installed in a shallow manhole, as shown in Specification 333006 Exhibit S-12A for a standard assembly and Exhibit S-12B for an offset assembly.
- 4.9.2.4 The Contractor shall furnish and install at no additional cost to the County all necessary fittings to make adjustments in the field for the installation of air release valves at all high points.

### **4.10 POLYETHYLENE TUBE ENCASEMENT/POLYWRAP**

- 4.10.1 All buried ductile iron pipe, valves, and fittings shall be polywrapped. Installation of polyethylene tube encasement shall be in accordance with Method A of ANSI/AWWA C105/A21.5 and as specified herein.
- 4.10.2 Raise a length of pipe at the side of the trench to a height of about three feet above ground level by means of hoisting equipment and a pipe sling or tongs.
- 4.10.3 Using a precut length of polyethylene tubing, two feet longer than length of pipe to be covered, slide plastic tubing over spigot end of the pipe up to the pipe sling or tongs. Bunch the excess of the plastic tubing near the sling or tongs.
- 4.10.4 Lower the pipe into the trench, joining the lowered length of pipe with that already in place. Shallow bell holes at the pipe joints must be made to facilitate overlapping of the polyethylene at the pipe joints.
- 4.10.5 Raise bell end of the pipe mechanically or by hand, clear of trench bottom. Slide plastic tube along balance of pipe length to the pipe bell. Leave surplus bunched at the bell for subsequent covering of the joint. Approximately one-foot of surplus should be provided at each end of pipe.
- 4.10.6 To cover the joined pipe joint pull the plastic tubing from the preceding length of pipe over the bell end of the pipe, fold around the spigot end of new pipe section and wrap with three circumferential turns of 1-1/2 inch wide polyethylene tape to seal and hold the film in place.
- 4.10.7 Pull the bunched polyethylene tubing on the new pipe barrel near spigot end over the first polyethylene wrap until it covers the joint, neatly folded behind the bell, seal and hold in place by three circumferential turns of 1-1/2 inch wide polyethylene adhesive tape. Use red tape around all restrained joints.
- 4.10.8 The polyethylene film covering the pipe will be loose. Excess material should be neatly drawn up around the pipe barrel, folded into an overlap on top of the pipe and held in place by means of pieces of the plastic tape at approximately three to five-foot intervals.
- 4.10.9 Repair any rips, punctures or other damage to the polyethylene with tape or by cutting open a short length of tube, wrapping it around the pipe and securing with tape.
- 4.10.10 Fittings such as pipe bends shall also be covered by use of the plastic tubing and plastic adhesive tape in much the same manner as the pipe.
- 4.10.11 Irregular-shaped appurtenances shall be covered by splitting a suitable length of the polyethylene tubing and using the resulting flat sheet with plastic tape to effect the covering of such items.



## **4.11 BACKFILL/COMPACTION**

- 4.11.1 Backfilling and compaction shall be conducted in a manner as to preclude subsequent settlement and provide adequate support for the surface treatment, pavement, pipelines, or structures to be placed thereon. All trenches shall be prepared per the requirements of Part 4.4 listed above. Also refer to Specification 333006, Exhibit S-15.
- 4.11.2 Backfill and bedding material shall be common fill material free from organic matter, muck or marl, and rock exceeding 2-1/2 inches in diameter, and shall not contain broken concrete, masonry, rubble or other similar materials. When unstable or unsuitable material is encountered replace with AASHTO soil classification A-1, A-2, or A-3.
- 4.11.3 Method of Compaction: The Contractor shall adopt compaction methods which will produce the degree of compaction specified herein without damage to the new or existing facilities. The degree of compaction specified in the following shall be considered the minimum allowable.
- 4.11.4 Backfilling Procedures: The backfilling procedures outlined in the following shall be for wastewater mains and related structures. The backfilling shall be done in three stages as follows:
- 4.11.4.1 In the first stage, the Contractor shall provide adequate compacted fill beneath the haunches of the pipe, using mechanical tampers suitable for this purpose. This compaction applies to the material placed beneath the haunches of the pipe and above any bedding material. Fill compacted by mechanical compactors shall be placed in 6-inch layers and thoroughly tamped over the entire surface.
- 4.11.4.2 In the second stage, the Contractor shall obtain a well-compacted bed and fill along the sides of the pipe and to a point of at least one foot above the top of the pipe. The width of backfill and compaction to be done under this second stage shall be the width of the portion of the trench having vertical sides; or, when no portion of the trench has vertical sides, it shall be to a width at least equal to three times the outside diameter of the pipe. Material to be placed in 6-inch layers (loose thickness).
- 4.11.4.3 In the third stage, the remainder of the trench shall be backfilled with suitable material in layers not to exceed 12-inch loose thickness and compacted.
- 4.11.5 Compaction Density: The trench backfill density for all stages shall be provided as follows:
- 4.11.5.1 Right-of-way line to right-of-way line, including all structures and railroad crossings: Compaction shall be 98 % of the maximum density as determined by AASHTO T-180 (ASTM D1557 - Modified Proctor) with no tolerance.
- 4.11.5.2 For outside of the right-of-way (but within maintenance easements): Compaction shall be 95% of the maximum density as determined by AASHTO T-180 (ASTM D1557 - Modified Proctor) with no tolerance.
- 4.11.6 Compaction Test Requirements
- 4.11.6.1 Compaction test results shall be submitted for all work.
- 4.11.6.2 Results of compaction tests must meet minimum requirements prior to proceeding with the next stage of the work.
- 4.11.6.3 For developer projects, one complete set of all test reports shall be submitted with the as-built package to the Site Plan and Subdivision Review Section upon project completion.
- 4.11.6.4 For County run projects, one complete set of all test reports shall be submitted with the as-built package to the Project Manager upon project completion.
- 4.11.6.5 The Contractor shall employ an independent testing laboratory, acceptable to the County and pay for all required tests.



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- 4.11.6.6 The laboratory shall submit one copy of the certified test reports, after testing in each phase, to the Construction Services Section in the Development Services Department, or the County Project Manager (as applicable), for approval.
- 4.11.6.7 In the second and third stage of backfilling, density tests shall be made every one foot vertically, staggered every 200 feet (minimum) horizontally. There shall be a minimum of one test (per vertical foot) between structures, and a minimum of one test per day.

### 4.12 FLUSHING AND CLEANING

- 4.12.1 All mains shall be cleaned and flushed to remove all sand and other foreign matter.
  - 4.12.1.1 The Contractor shall be responsible for developing a flushing plan to be submitted to the Engineer of Record for approval with the shop drawings.
  - 4.12.1.2 The Contractor shall dispose of all water used for flushing without causing a nuisance or property damage.
  - 4.12.1.3 Any permits required for the disposal of flushing water shall be the responsibility of the Contractor.
- 4.12.2 High pressure cleaning of lined DI pipe and fittings is not to be used.
- 4.12.3 Flushing water used by the Contractor shall be taken from an approved metered source. Flushing water shall be at Contractor's expense.
- 4.12.4 Pipeline shall be cleaned with a "pig", of an appropriate material for the pipeline to be cleaned, so as not to damage the interior lining of the pipeline. The Contractor shall be responsible to install and remove appropriate connections to accomplish the required pipeline "pigging".
- 4.12.5 Temporary plugs or caps shall be installed on new mains until the pressure and leakage tests are completed. Upon satisfactory completion of the tests the caps or plugs shall be removed and the connections made to the existing mains.

### 4.13 HYDROSTATIC AND LEAKAGE TEST

- 4.13.1 The force main shall be tested in sections between valves. The total length of pipe for any single test shall be 2,000 feet. Testing shall be done immediately after installation and backfilling has been completed.
- 4.13.2 The piping shall be tested in sections, thereby, testing each valve for secure closure.
- 4.13.3 The mains shall be tested in accordance with, the latest revision of AWWA C600 (for Ductile Iron) and C605 (for PVC) under an average hydrostatic pressure of not less than 150 psig, using a 300 psig gage, for a minimum of two hours. Pressure shall be maintained until all sections under testing have been checked for evidence of leakage.
- 4.13.4 While the system is being filled with water, air shall be carefully and completely exhausted. If permanent air vents are not located at all the high points, the Contractor shall install corporation stops or fittings and valves at such points so the air can be expelled as the pipe system is slowly filled.
- 4.13.5 The test pressure shall not vary by more than five psi for the duration of the test. The rate of loss shall not exceed that specified in sections 4.13.13 or 4.13.14. Visible leaks shall be corrected regardless of total leakage shown by test.



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- 4.13.6 All pumps, gauges and measuring devices shall be furnished, installed and operated by the Contractor and all such equipment and devices and their installation shall be approved by the County Inspector.
- 4.13.7 Water for testing and flushing shall be potable or reclaimed water provided by the Contractor, at no cost to the County, from a source approved by the County. Flow velocity during line filling should not exceed 2 feet per second.
- 4.13.8 The quantity of water used for testing, which shall be compared to the allowable quantity, shall be measured by pumping from a calibrated container approved by the County Inspector.
- 4.13.9 All restrained sections of the buried main shall be completely backfilled before such sections are tested.
- 4.13.10 All pressure lines shall be tested. All pressure and leakage testing shall be done in the presence of the County Inspector and the Engineer of Record or his designated representative. Pressure and leakage testing is considered a “hold” point and requires the sign off of the County Inspector.
- 4.13.11 When leakage occurs in excess of the specified amount, defective pipe, pipe joints or other appurtenances shall be located and repaired at the expense of the Contractor. If the defective portions cannot be located, the Contractor, at his own expense, shall remove and reconstruct as much of the original work as necessary to obtain a force main within the allowable leakage limits upon retesting.
- 4.13.12 If the Contractor elects to perform hydrostatic testing against valves in an existing distribution system, he does so at his own risk and will bear the cost of any damage to the existing valve, piping system, private or public property, or the new pipeline under test.
- 4.13.13 Allowable Limits for Leakage in Ductile Iron Pipe
  - 4.13.13.1 The hydrostatic pressure test shall be performed as specified herein and no installation, or section thereof, will be acceptable until the leakage is less than the number of gallons per hour as determined by the formula:

$$L = \frac{SD(P)^{1/2}}{148,000}$$

in which:

- L = Testing Allowance (makeup water), in gallons per hour
- S = Length of pipe in feet being tested
- D = Nominal diameter of pipe, in inches
- P = Average test pressure during the test, in psig

- 4.13.13.2 Water shall be supplied to the main during the test period as required to maintain the test pressure as specified.
- 4.13.13.3 The quantity used, which shall be compared to the previous allowable quantity, shall be measured by pumping from a calibrated container. A 5/8-inch meter installed on the discharge side of the pump may be used to measure the leakage for large mains when approved by the County Inspector All hydrostatic leakage tests shall be recorded on the form(s) attached in Part 5.
- 4.13.14 Allowable Limits for Leakage in PVC Pipe
  - 4.13.14.1 The hydrostatic pressure test shall be performed as specified herein and no installation



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or section thereof, will be acceptable until the leakage is less than the number of gallons per hour as determined by the formula:

$$(1) \quad Q = \frac{LD(P)^{1/2}}{148,000} \qquad \text{OR} \qquad (2) \quad Q = \frac{ND(P)^{1/2}}{7,400}$$

in which:

- Q = Quantity of makeup water, in gallons per hour
- N = Number of joints in the tested line (pipe & fittings)
- L = Length of pipe section being tested, in feet
- D = Nominal diameter of pipe in inches.
- P = Average test pressure during the test, in psig

NOTE: Equation (2) is for gasketed pipe in 20-foot lengths.

4.13.14.2 Water shall be supplied to the main during the test period as required to maintain the test pressure as specified. The quantity used, which shall be compared to the previous allowable quantity, shall be measured by pumping from a calibrated container. A 5/8-inch meter installed on the discharge side of the pump may be used to measure the leakage for large mains when approved by the County Inspector All hydrostatic leakage tests shall be recorded on the form(s) attached in Part 5.

### 4.14 CONNECTIONS TO EXISTING SYSTEM

- 4.14.1 The Contractor shall supply a connection and procedure schedule to the County, for approval, two weeks prior to the proposed connection date. Contractor shall be responsible for the coordination of any/all of the existing private pump stations shut-down After approval of the schedule, the County will be responsible for shutting down the County owned pump stations or valves as applicable. The Contractor shall then make the required connection as quickly as possible. The Contractor is responsible to coordinate and provide any and all pumping, and/or removal of effluent at connection points to existing mains and at affected pump/lift stations (County owned or private) during wastewater connection operations. Contractor is also responsible for any trucking of effluent and the proper disposal of wastewater, and any other work required to maintain existing services until and during transfer to the new service.
- 4.14.2 The Contractor is responsible to provide, operate, and maintain all wastewater flow by-passes required to complete the project. Coordinate and obtain procedure approval from the Engineer and Public Utility representative at least 48 hours prior to implementation.
- 4.14.3 All connections to existing mains shall be made under the direction supervision of Hillsborough County PUD after the Contractor has coordinated with and received approval from the County (approval shall be obtained through DSD, or the County Project Manager, as applicable). At all new points of connections, a tee or cross with the appropriate isolation valves shall be installed
- 4.14.4 Valves on existing mains will be operated by Hillsborough County PUD personnel or under their direct supervision.
- 4.14.5 If a tee and isolation valve cannot be cut in then the following procedures shall be followed.  
Tapping a Force Main
  - 4.14.5.1 No size on size taps are allowed.
  - 4.14.5.2 The Contractor shall submit a request to the Service Availability Team, Customer Service Section of Hillsborough County PUD to schedule a tap, and pay the appropriate



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- tapping fee. The request shall be made a minimum of 48 hours prior to the proposed tie-in.
- 4.14.5.3 The Contractor shall furnish, install and pressure test the tapping sleeve and valves to the existing force main.
  - 4.14.5.4 For all taps up through 12 inches, Hillsborough County PUD personnel will furnish the necessary tapping machine and tools, and will perform the tap.
  - 4.14.5.5 For taps larger than 12 inches, the Contractor shall furnish the tapping machine and tools, and shall perform the tap under Hillsborough County PUD supervision. All taps shall be witnessed by the County Inspector.
  - 4.14.5.6 Prior to the tap the Contractor shall assemble all materials, tools, equipment, labor and supervision necessary to make the connection.
  - 4.14.5.7 The Contractor shall excavate and maintain a dry and safe working pit of sufficient size to facilitate the inspection and tapping of the line.
  - 4.14.5.8 The Contractor will locate the main, and shall pressure test the tapping sleeve and valve to 150 psig, or 10 psig above the pressure in the main being tapped, whichever is greater.
  - 4.14.5.9 The Contractor will maintain the pressure on the sleeve for 10 minutes at zero (0) pressure loss.
  - 4.14.5.10 When the Contractor is required to make the tap, they shall make the tap while the main is in service using standard tapping techniques as approved by the County
    - a) The force main shall be tapped in such a manner that the operation of the main in service is not disturbed.
    - b) The Contractor shall obtain the approval of Public Utilities Department for methodology and subcontractor personnel prior to initiating any tap.
  - 4.14.5.11 The Contractor shall be responsible for properly backfilling the work pit after the work is complete.
  - 4.14.5.12 The tapping valve, isolation plug valve, and sleeve become the property of the County upon successful completion of the tap.
- 4.14.6 Verification is required that each tap has been performed onto the intended pipe. The coupon is to be removed and inspected. The valve is to be cracked open to verify pressure and type of liquid being expelled. At the County's option a pressure test valve may be required to be installed at no cost to the County.



**PART 5.0 PRESSURE TESTING FORMS**  
**PRESSURE TEST REPORT**

PROJECT: \_\_\_\_\_

ENGINEER OF RECORD: \_\_\_\_\_

CONTRACTOR: \_\_\_\_\_

SERVICE REQUEST NO: \_\_\_\_\_ DATE: \_\_\_\_\_ INSPECTOR: \_\_\_\_\_

\_\_\_\_\_ WATER PRESSURE TEST                      \_\_\_\_\_ FORCE MAIN PRESSURE  
\_\_\_\_\_ RECLAIMED MAIN PRESSURE

CALCULATIONS BASED ON AWWA. STD. C600, SECTION 5

$$L = \frac{S D (P)^{1/2}}{148,000}$$

WHERE:        L = TESTING ALLOWANCE (MAKEUP WATER) IN GALS. PER HOUR  
                  S = LENGTH OF PIPE TESTED, IN FEET  
                  D = NOMINAL DIAMETER OF PIPE IN INCHES  
                  P = AVERAGE TEST PRESSURE (PSIG)

CALCULATIONS:

$$L = \frac{[ \quad ] [ \quad ] ( \quad )^{1/2}}{148,000} = \quad \text{GALS./HR. ALLOWED}$$

$$L = \frac{[ \quad ] [ \quad ] ( \quad )^{1/2}}{148,000} = \quad \text{GALS./HR. ALLOWED}$$

$$L = \frac{[ \quad ] [ \quad ] ( \quad )^{1/2}}{148,000} = \quad \text{GALS./HR. ALLOWED}$$



## ALLOWABLE LEAKAGE TEST REPORT FOR DUCTILE IRON PIPE

PROJECT NAME: \_\_\_\_\_ PROJ. NO.: \_\_\_\_\_

ENGINEER OF RECORD NAME: \_\_\_\_\_

COMPANY: \_\_\_\_\_

CONTRACTOR REPRESENTATIVE NAME: \_\_\_\_\_

COMPANY NAME: \_\_\_\_\_

SERVICE REQUEST NO.: \_\_\_\_\_ DATE: \_\_\_\_\_

INSPECTOR: \_\_\_\_\_

\_\_\_\_\_ WATER PRESSURE TEST                      \_\_\_\_\_ FORCEMAIN PRESSURE TEST

\_\_\_\_\_ RECLAIMED MAIN PRESSURE

CALCULATIONS BASED ON AWWA C600

$$L = \frac{S D (P)^{1/2}}{148,000}$$

WHERE:        L = TESTING ALLOWANCE (MAKEUP WATER) IN GALS. PER HOUR.  
                  S = LENGTH OF PIPE TESTED, IN FEET  
                  D = NOMINAL DIAMETER OF PIPE IN INCHES  
                  P = AVERAGE TEST PRESSURE (PSIG)

CALCULATIONS:

$$L = \frac{[ \quad ] [ \quad ] ( \quad )^{1/2}}{148,000} = \text{_____ GALS./HR. ALLOWED}$$
$$\text{_____ GALS.HR. X 2 HR.} = \text{_____ GALS. ALLOWED LOST}$$

FIELD MEASUREMENTS:

START TEST @ \_\_\_\_\_ PRESSURE = \_\_\_\_\_ # READING = \_\_\_\_\_

FINISH TEST @ \_\_\_\_\_ PRESSURE = \_\_\_\_\_ # READING = \_\_\_\_\_

LOSS = \_\_\_\_\_ # GALLONS = \_\_\_\_\_

PEOPLE PRESENT:

TEST RESULTS AND COMMENTS:



# ALLOWABLE LEAKAGE TEST REPORT FOR POLYVINYL CHLORIDE PIPE

PROJECT NAME: \_\_\_\_\_ PROJ. NO.: \_\_\_\_\_

ENGINEER OF RECORD NAME: \_\_\_\_\_

COMPANY: \_\_\_\_\_

CONTRACTOR REPRESENTATIVE NAME: \_\_\_\_\_

COMPANY NAME: \_\_\_\_\_

SERVICE REQUEST NO.: \_\_\_\_\_ DATE: \_\_\_\_\_

INSPECTOR: \_\_\_\_\_

\_\_\_\_\_ WATER PRESSURE TEST                      \_\_\_\_\_ FORCEMAIN PRESSURE TEST  
\_\_\_\_\_ RECLAIMED MAIN PRESSURE

CALCULATION (1) BASED ON AWWA C605  
CALCULATION (2) BASED ON UNI-BELL EQUATION 99 FOR GASKETED PIPE IN 20 FT LENGTHS.

$$(1) \quad Q = \frac{L D (P)^{1/2}}{148,000} \qquad (2) \quad Q = \frac{N D (P)^{1/2}}{7,400}$$

WHERE:            Q = ALLOWABLE LEAKAGE IN GALS. PER HOUR.  
                      L = LENGTH OF PIPE BEING TESTED, IN FEET  
                      N = NUMBER OF JOINTS IN THE TESTED LINE (PIPE AND FITTINGS)  
                      D = NOMINAL DIAMETER OF PIPE IN INCHES  
                      P = AVERAGE TEST PRESSURE (PSIG)

CALCULATIONS:

$$Q = \frac{[ \quad ] [ \quad ] ( \quad )^{1/2}}{148,000} = \text{_____ GALS./HR. ALLOWED}$$
$$\text{GALS.HR. X 2 HR.} = \text{_____ GALS. ALLOWED LOST}$$

FIELD MEASUREMENTS:

START TEST @ \_\_\_\_\_ PRESSURE = \_\_\_\_\_ # READING = \_\_\_\_\_  
FINISH TEST @ \_\_\_\_\_ PRESSURE = \_\_\_\_\_ # READING = \_\_\_\_\_  
LOSS = \_\_\_\_\_ # GALLON = \_\_\_\_\_ #

PEOPLE PRESENT:

TEST RESULTS AND COMMENTS: